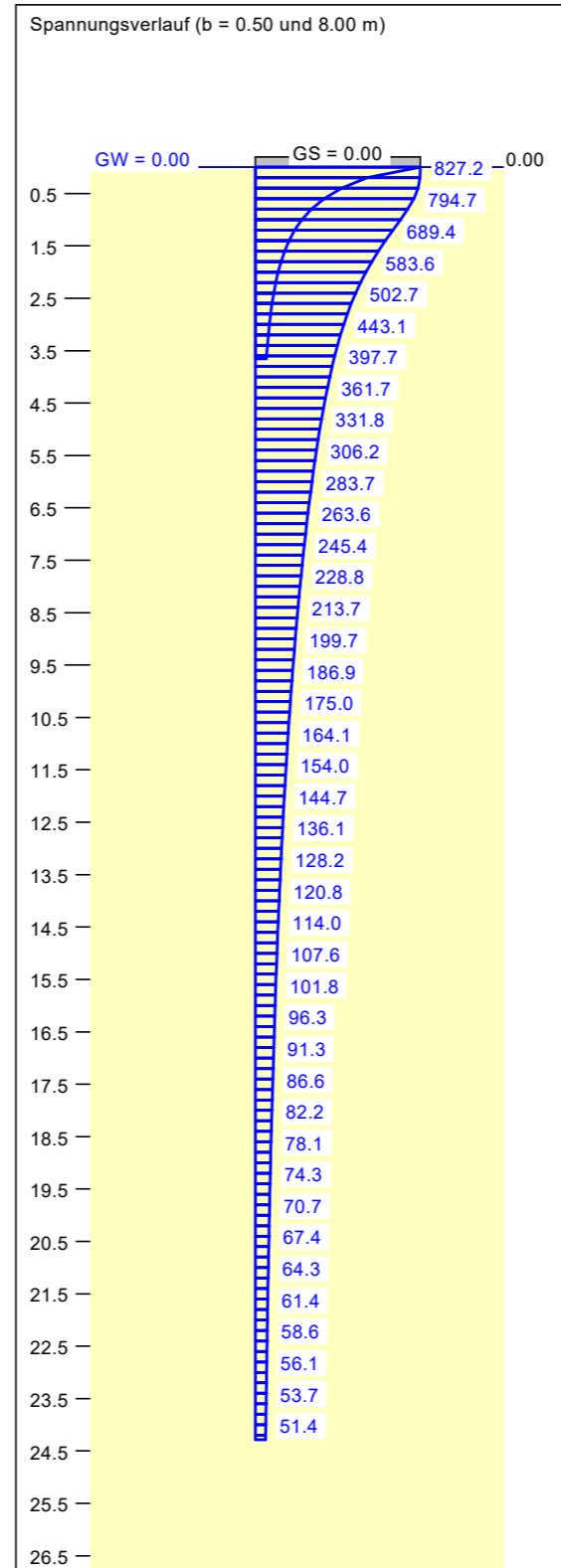
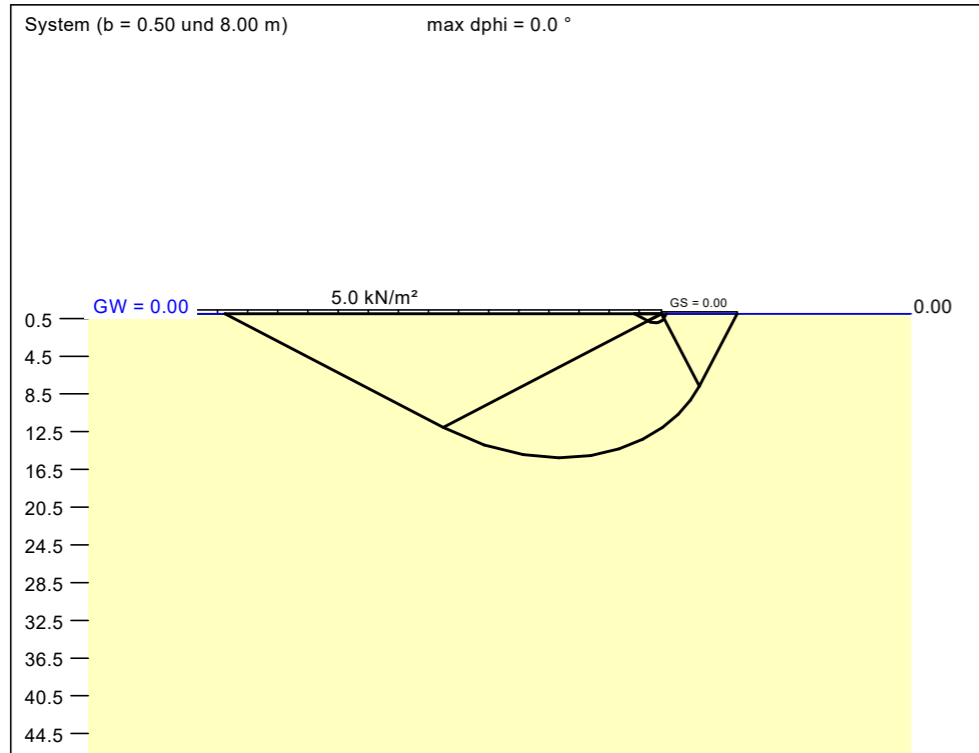


Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\varphi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
	20.0	10.0	35.0	0.0	80.0	0.00	Kies



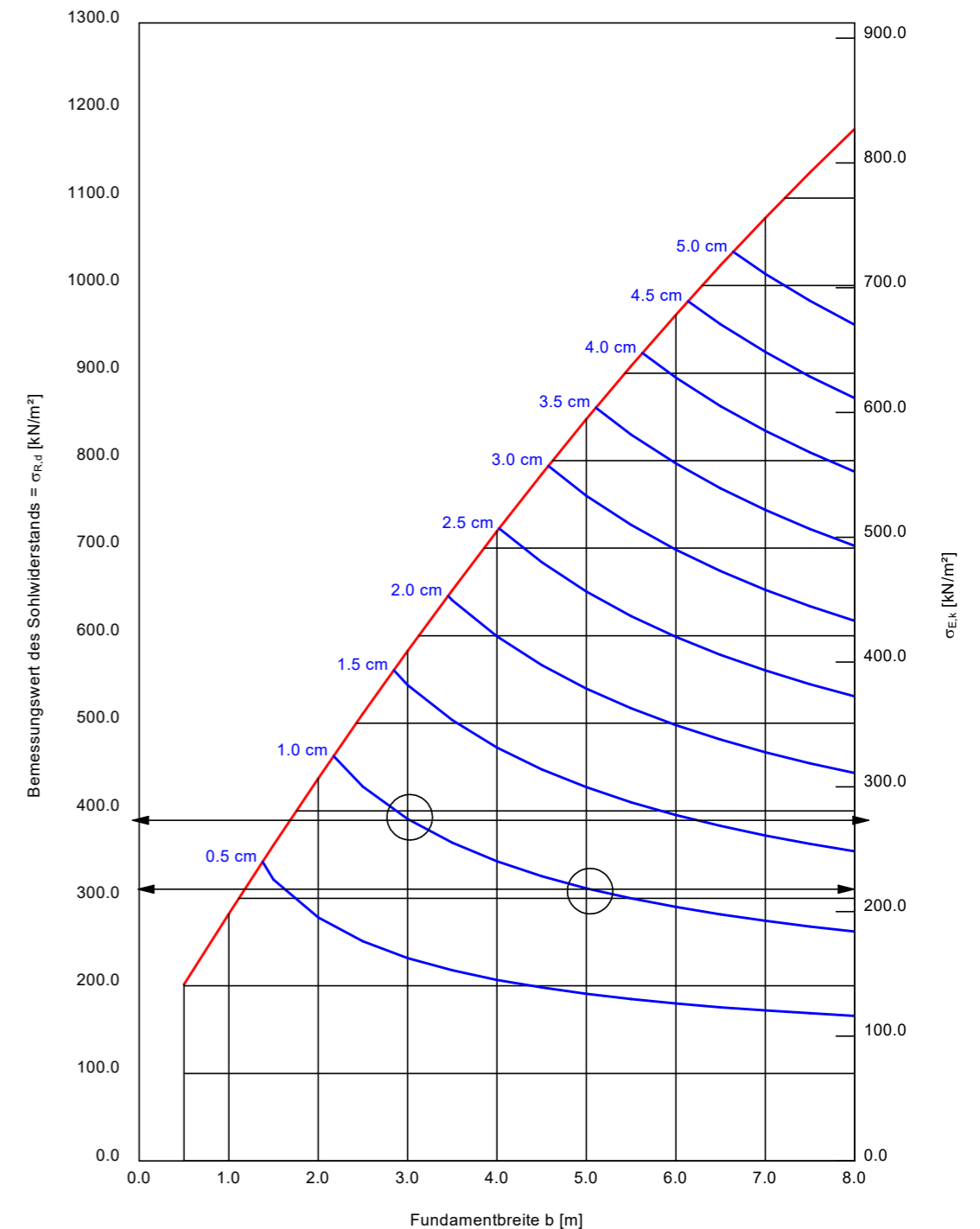
Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 11.00 m)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 Gründungssohle = 0.00 m  
 Grundwasser = 0.00 m  
 Vorbelastung = 30.0 kN/m<sup>2</sup>  
 Grenztiefe mit p = 20.0 %  
 Grenztiefen spannungsvariabel bestimmt

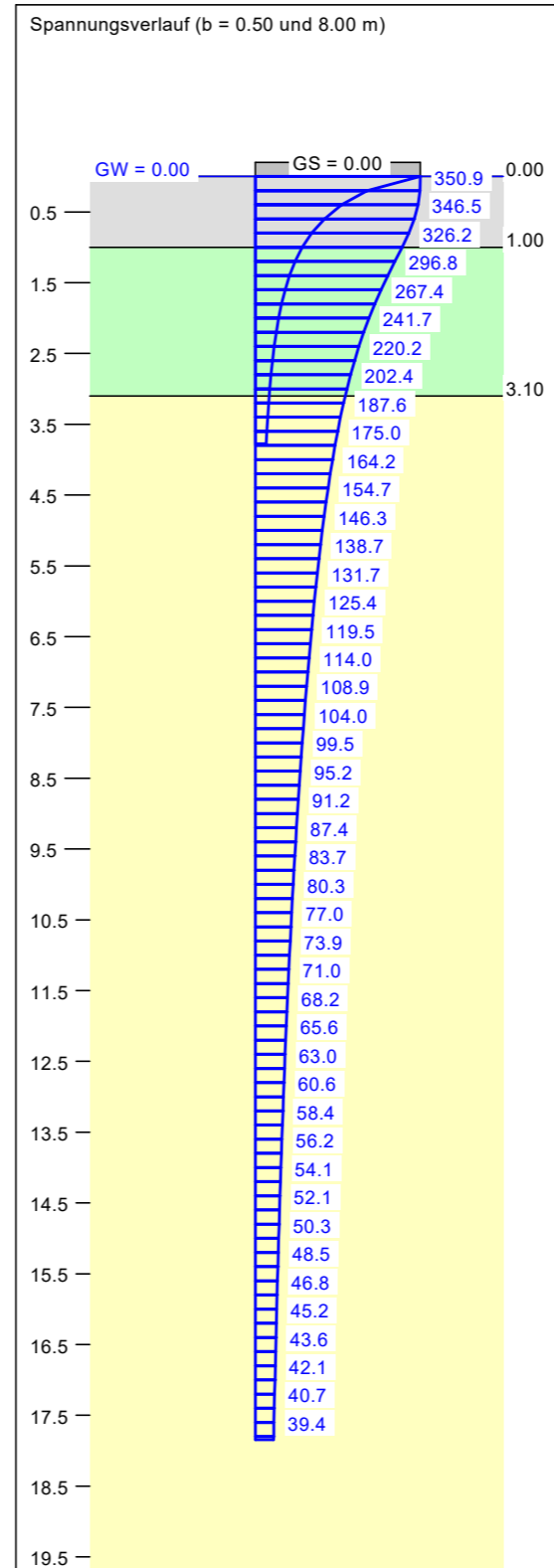
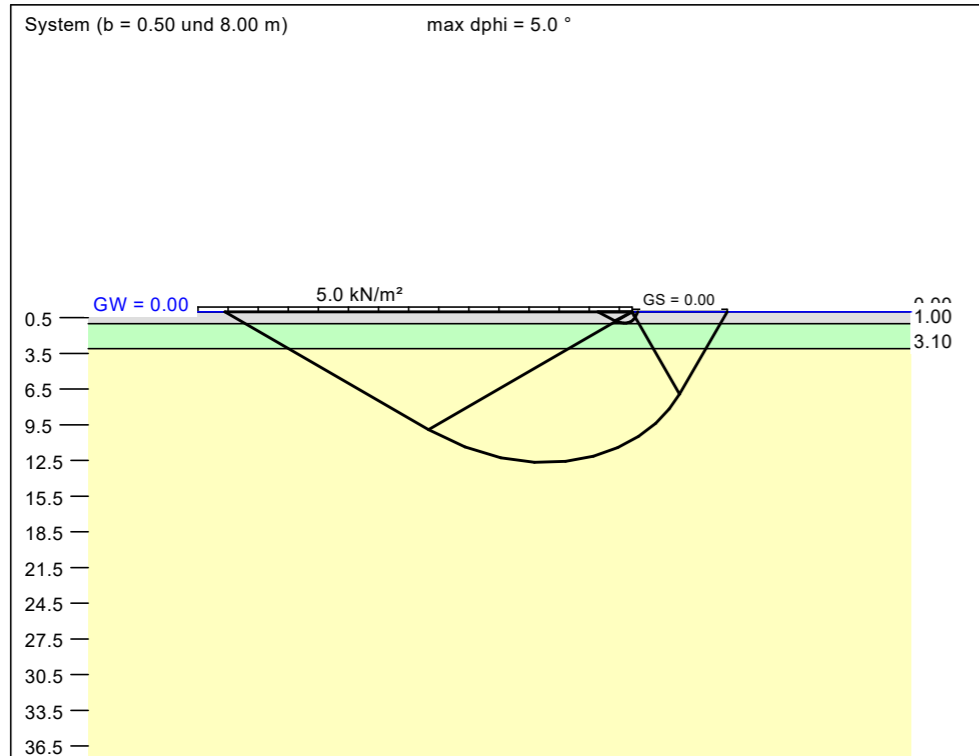
— Sohlendruck  
 — Setzungen

a	b	$\sigma_{R,d}$	$R_{n,d}$	$\sigma_{E,k}$	s	cal $\varphi$	cal c	$\gamma_2$	$\sigma_0$	$t_g$	UK LS
[m]	[m]	[kN/m <sup>2</sup> ]	[kN/m]	[kN/m <sup>2</sup> ]	[cm]	[°]	[kN/m <sup>2</sup> ]	[kN/m <sup>3</sup> ]	[kN/m <sup>2</sup> ]	[m]	[m]
11.00	0.50	201.7	100.8	141.5	0.12 *	35.0	0.00	10.00	5.00	3.65	0.95
11.00	1.00	282.2	282.2	198.1	0.32 *	35.0	0.00	10.00	5.00	5.89	1.91
11.00	1.50	360.6	540.9	253.0	0.58 *	35.0	0.00	10.00	5.00	7.90	2.86
11.00	2.00	436.8	873.5	306.5	0.89 *	35.0	0.00	10.00	5.00	9.72	3.82
11.00	2.50	510.7	1276.8	358.4	1.24 *	35.0	0.00	10.00	5.00	11.40	4.77
11.00	3.00	582.5	1747.4	408.7	1.63 *	35.0	0.00	10.00	5.00	12.94	5.72
11.00	3.50	652.0	2282.0	457.5	2.04 *	35.0	0.00	10.00	5.00	14.38	6.68
11.00	4.00	719.3	2877.4	504.8	2.48 *	35.0	0.00	10.00	5.00	15.73	7.63
11.00	4.50	784.5	3530.2	550.5	2.93 *	35.0	0.00	10.00	5.00	17.00	8.58
11.00	5.00	847.4	4237.1	594.7	3.40 *	35.0	0.00	10.00	5.00	18.20	9.54
11.00	5.50	908.2	4994.9	637.3	3.88 *	35.0	0.00	10.00	5.00	19.33	10.49
11.00	6.00	966.7	5800.2	678.4	4.37 *	35.0	0.00	10.00	5.00	20.42	11.45
11.00	6.50	1023.0	6649.7	717.9	4.86 *	35.0	0.00	10.00	5.00	21.45	12.40
11.00	7.00	1077.2	7540.1	755.9	5.36 *	35.0	0.00	10.00	5.00	22.43	13.35
11.00	7.50	1129.1	8468.1	792.3	5.85 *	35.0	0.00	10.00	5.00	23.38	14.31
11.00	8.00	1178.8	9430.4	827.2	6.35 *	35.0	0.00	10.00	5.00	24.28	15.26

\* Vorbelastung = 30.0 kN/m<sup>2</sup>  
 $\sigma_{E,k} = \sigma_{0f,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{0f,k} / (1.40 \cdot 1.43) = \sigma_{0f,k} / 1.99$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50



Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\phi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
	20.0	10.0	35.0	0.0	80.0	0.00	Bodenersatzkörper
	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer
	20.0	10.0	35.0	0.0	80.0	0.00	Kies



Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 12.50 m)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 $\sigma_{R,d}$  auf 500.00 kN/m<sup>2</sup> begrenzt  
 Gründungssohle = 0.00 m  
 Grundwasser = 0.00 m  
 Vorbelastung = 30.0 kN/m<sup>2</sup>  
 Grenztiefe mit  $p = 20.0$  %  
 Grenztiefen spannungsvariabel bestimmt

— Setzungen

— Sohlendruck

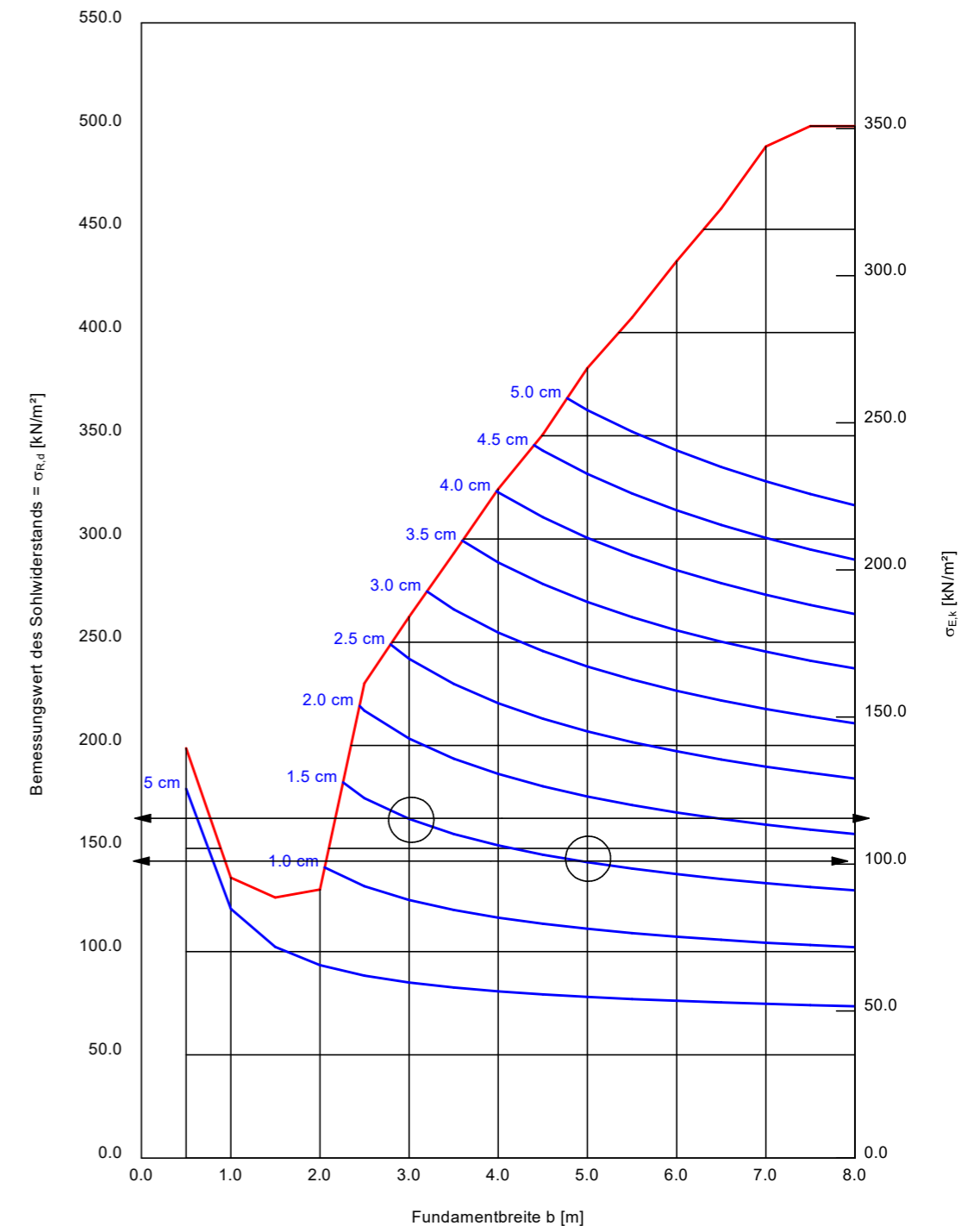
a [m]	b [m]	$\sigma_{R,d}$ [kN/m <sup>2</sup> ]	$R_{n,d}$ [kN/m]	$\sigma_{E,k}$ [kN/m <sup>2</sup> ]	s [cm]	cal $\phi$ [°]	cal c [kN/m <sup>2</sup> ]	$\gamma_2$ [kN/m <sup>3</sup> ]	$\sigma_0$ [kN/m <sup>2</sup> ]	$t_g$ [m]	UK LS [m]
12.50	0.50	198.6	99.3	139.4	0.57 *	34.9 **	0.00	10.00	5.00	3.77	0.95
12.50	1.00	135.8	135.8	95.3	0.60 *	29.9 **	0.00	9.78	5.00	4.04	1.59
12.50	1.50	126.3	189.4	88.6	0.71 *	27.9 **	0.00	9.60	5.00	4.54	2.22
12.50	2.00	130.1	260.2	91.3	0.88 *	27.0 **	0.00	9.48	5.00	5.20	2.86
12.50	2.50	230.0	575.0	161.4	2.16 *	29.9 **	0.00	9.46	5.00	7.99	3.96
12.50	3.00	262.2	786.7	184.0	2.76 *	30.0 **	0.00	9.51	5.00	9.20	4.75
12.50	3.50	293.1	1025.7	205.7	3.38 *	30.0 **	0.00	9.55	5.00	10.34	5.54
12.50	4.00	324.2	1296.8	227.5	4.03 *	30.0 **	0.00	9.59	5.00	11.43	6.34
12.50	4.50	350.7	1578.0	246.1	4.62 *	29.9 **	0.00	9.63	5.00	12.41	7.11
12.50	5.00	382.7	1913.6	268.6	5.33 *	30.0 **	0.00	9.66	5.00	13.44	7.92
12.50	5.50	407.2	2239.6	285.8	5.93 *	29.9 **	0.00	9.68	5.00	14.31	8.70
12.50	6.00	434.6	2607.4	305.0	6.59 *	29.9 **	0.00	9.70	5.00	15.20	9.49
12.50	6.50	459.9	2989.7	322.8	7.24 *	29.9 **	0.00	9.72	5.00	16.03	10.28
12.50	7.00	490.1	3430.9	343.9	7.98 *	30.0 **	0.00	9.74	5.00	16.91	11.09
12.50	7.50	500.0	3750.0	350.9	8.35 *	30.0 **	0.00	9.75	5.00	17.46	11.87
12.50	8.00	500.0	4000.0	350.9	8.54 *	29.9 **	0.00	9.77	5.00	17.84	12.64

\* Vorbelastung = 30.0 kN/m<sup>2</sup>

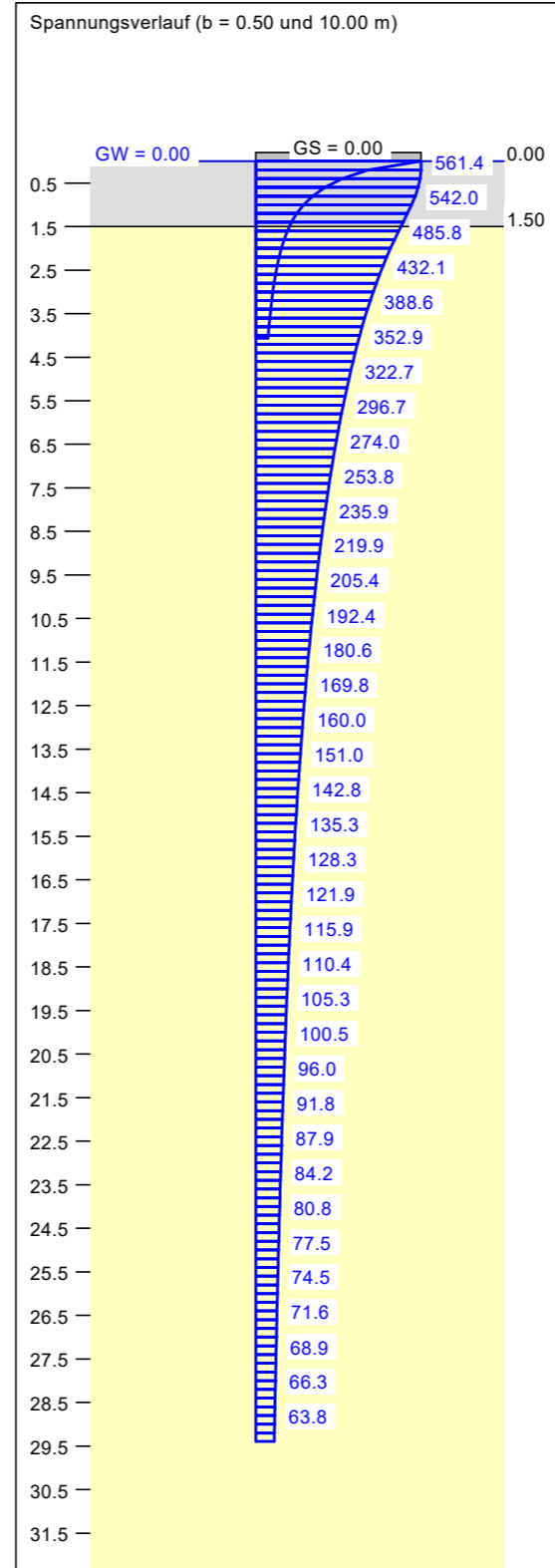
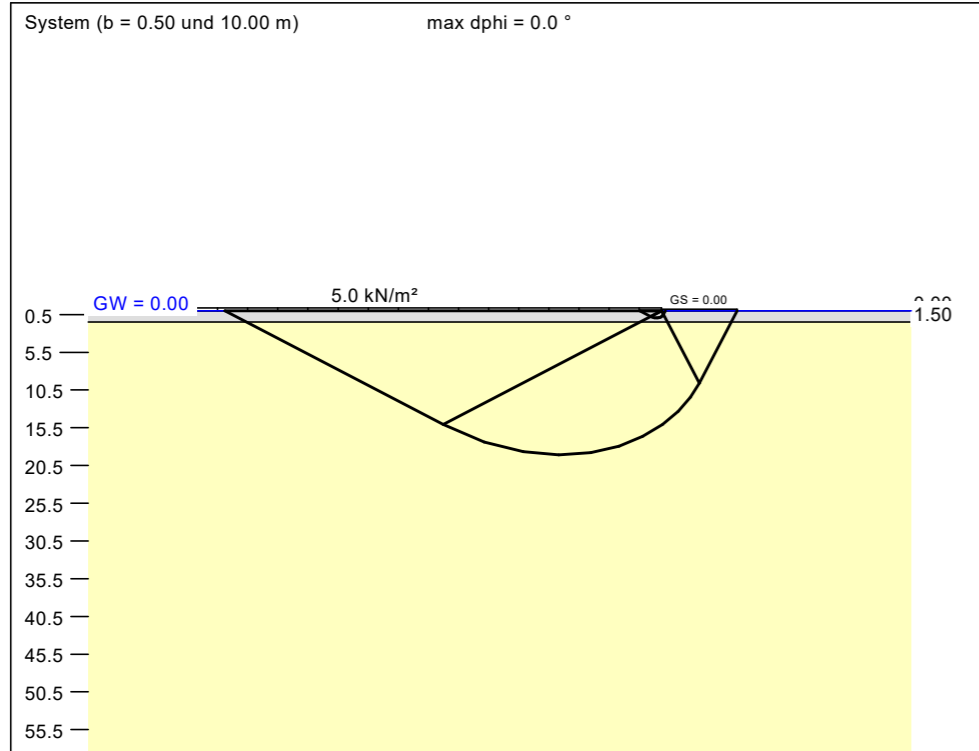
\*\* phi wegen 5° Bedingung abgemindert

$\sigma_{E,k} = \sigma_{of,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{of,k} / (1.40 \cdot 1.43) = \sigma_{of,k} / 1.99$  (für Setzungen)

Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50



Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\varphi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
	20.0	10.0	35.0	0.0	80.0	0.00	Bodenersatzkörper
	20.0	10.0	35.0	0.0	80.0	0.00	Kies



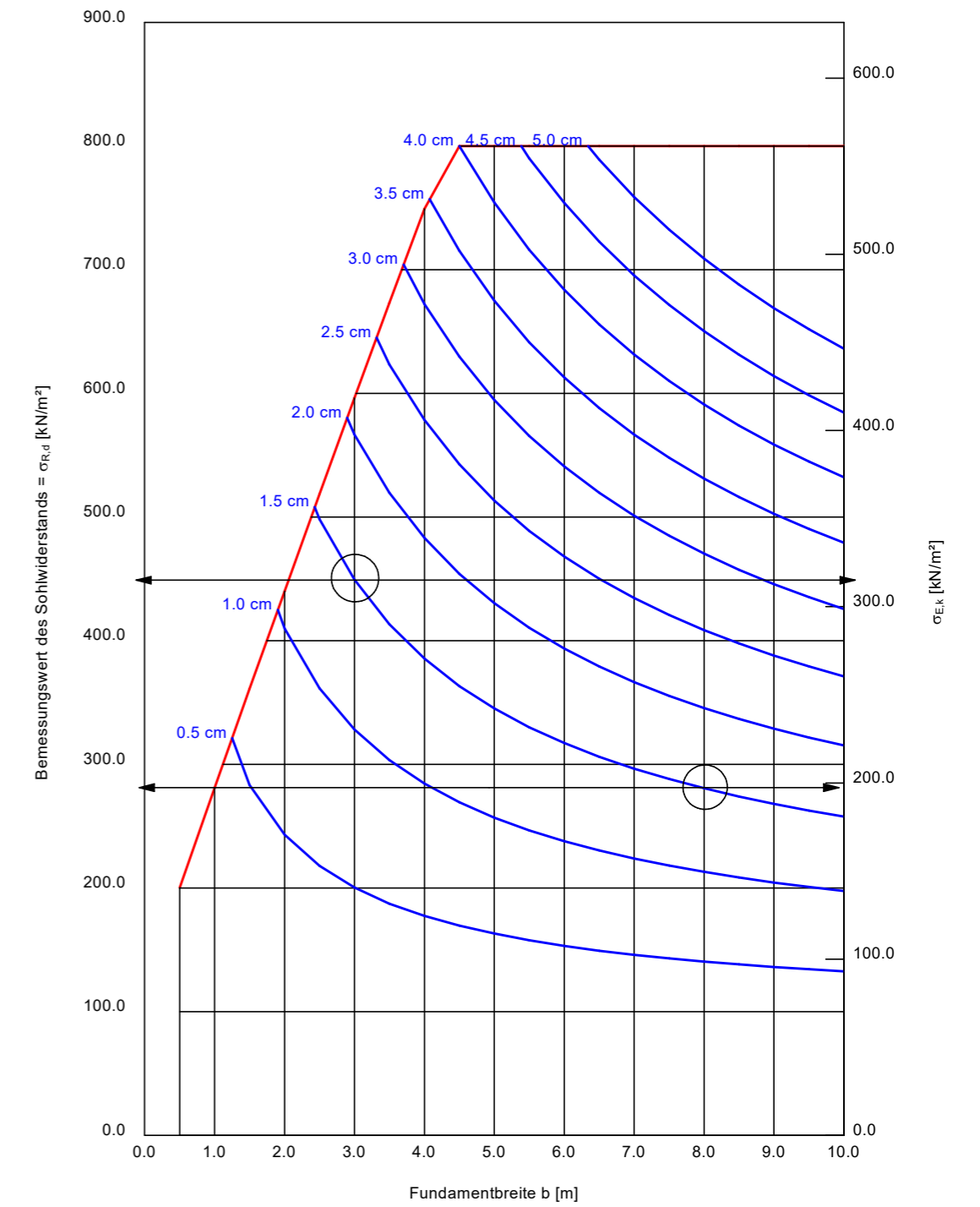
Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 32.00 m)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 $\sigma_{R,d}$  auf 800.00 kN/m<sup>2</sup> begrenzt  
 Gründungssohle = 0.00 m  
 Grundwasser = 0.00 m  
 Vorbelastung = 30.0 kN/m<sup>2</sup>  
 Grenztiefe mit p = 20.0 %  
 Grenztiefen spannungsvariabel bestimmt

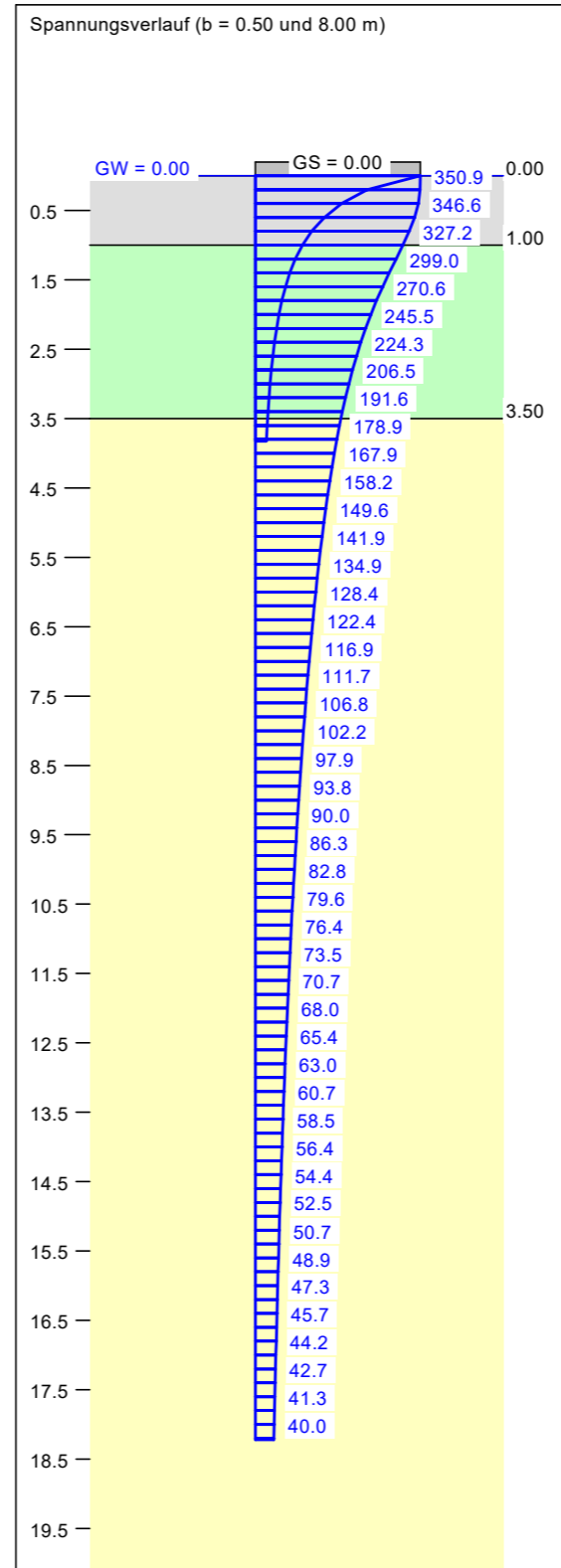
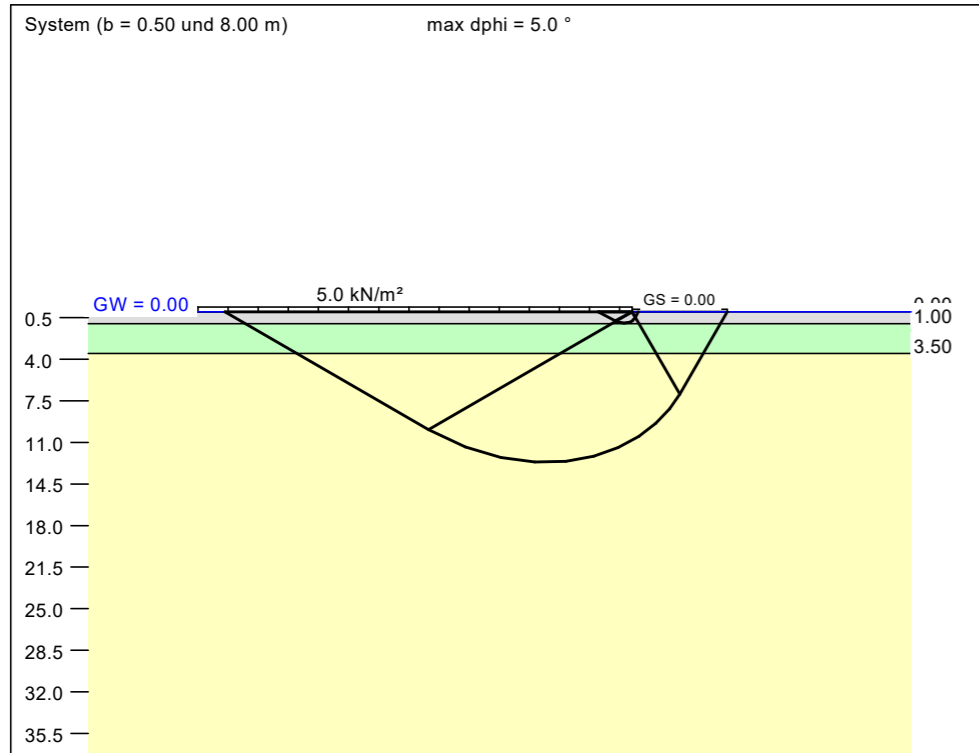
— Setzungen  
 — Sohlendruck

a [m]	b [m]	$\sigma_{R,d}$ [kN/m <sup>2</sup> ]	$R_{n,d}$ [kN/m]	$\sigma_{E,k}$ [kN/m <sup>2</sup> ]	s [cm]	cal $\varphi$ [°]	cal c [kN/m <sup>2</sup> ]	$\gamma_2$ [kN/m <sup>3</sup> ]	$\sigma_{\bar{0}}$ [kN/m <sup>2</sup> ]	$t_g$ [m]	UK LS [m]
32.00	0.50	200.4	100.2	140.6	0.13 *	35.0	0.00	10.00	5.00	4.06	0.95
32.00	1.00	281.1	281.1	197.2	0.36 *	35.0	0.00	10.00	5.00	6.72	1.91
32.00	1.50	361.0	541.5	253.3	0.69 *	35.0	0.00	10.00	5.00	9.13	2.86
32.00	2.00	440.2	880.4	308.9	1.09 *	35.0	0.00	10.00	5.00	11.40	3.82
32.00	2.50	518.6	1296.5	363.9	1.58 *	35.0	0.00	10.00	5.00	13.59	4.77
32.00	3.00	596.3	1788.8	418.4	2.13 *	35.0	0.00	10.00	5.00	15.70	5.72
32.00	3.50	673.2	2356.1	472.4	2.74 *	35.0	0.00	10.00	5.00	17.75	6.68
32.00	4.00	749.3	2997.3	525.8	3.42 *	35.0	0.00	10.00	5.00	19.73	7.63
32.00	4.50	800.0	3600.0	561.4	4.00 *	35.0	0.00	10.00	5.00	21.36	8.58
32.00	5.00	800.0	4000.0	561.4	4.29 *	35.0	0.00	10.00	5.00	22.31	9.54
32.00	5.50	800.0	4400.0	561.4	4.57 *	35.0	0.00	10.00	5.00	23.20	10.49
32.00	6.00	800.0	4800.0	561.4	4.83 *	35.0	0.00	10.00	5.00	24.03	11.45
32.00	6.50	800.0	5200.0	561.4	5.08 *	35.0	0.00	10.00	5.00	24.82	12.40
32.00	7.00	800.0	5600.0	561.4	5.33 *	35.0	0.00	10.00	5.00	25.57	13.35
32.00	7.50	800.0	6000.0	561.4	5.56 *	35.0	0.00	10.00	5.00	26.28	14.31
32.00	8.00	800.0	6400.0	561.4	5.78 *	35.0	0.00	10.00	5.00	26.96	15.26
32.00	8.50	800.0	6800.0	561.4	6.00 *	35.0	0.00	10.00	5.00	27.60	16.22
32.00	9.00	800.0	7200.0	561.4	6.21 *	35.0	0.00	10.00	5.00	28.22	17.17
32.00	9.50	800.0	7600.0	561.4	6.41 *	35.0	0.00	10.00	5.00	28.82	18.12
32.00	10.00	800.0	8000.0	561.4	6.61 *	35.0	0.00	10.00	5.00	29.39	19.08

\* Vorbelastung = 30.0 kN/m<sup>2</sup>  
 $\sigma_{E,k} = \sigma_{0f,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{0f,k} / (1.40 \cdot 1.43) = \sigma_{0f,k} / 1.99$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50



Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\phi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
■	20.0	10.0	35.0	0.0	80.0	0.00	Bodenersatzkörper
■	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer Schluff
■	20.0	10.0	35.0	0.0	80.0	0.00	Kies



Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 13.50 m)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 $\sigma_{R,d}$  auf 500.00 kN/m<sup>2</sup> begrenzt  
 Gründungssohle = 0.00 m  
 Grundwasser = 0.00 m  
 Vorbelastung = 30.0 kN/m<sup>2</sup>  
 Grenztiefe mit p = 20.0 %  
 Grenztiefen spannungsvariabel bestimmt

— Setzungen

— Sohldruck

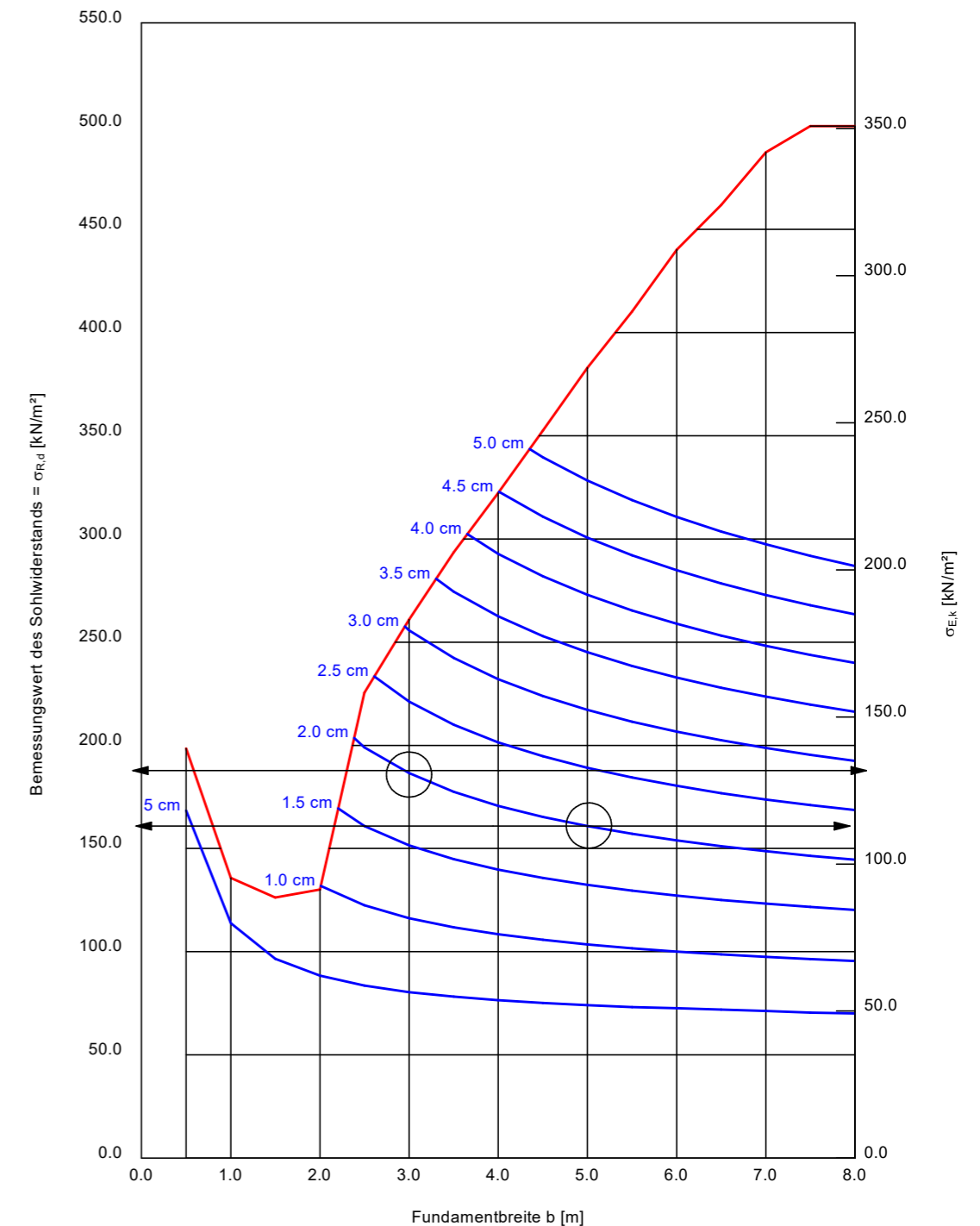
a [m]	b [m]	$\sigma_{R,d}$ [kN/m <sup>2</sup> ]	$R_{n,d}$ [kN/m]	$\sigma_{E,k}$ [kN/m <sup>2</sup> ]	s [cm]	cal $\phi$ [°]	cal c [kN/m <sup>2</sup> ]	$\gamma_2$ [kN/m <sup>3</sup> ]	$\sigma_0$ [kN/m <sup>2</sup> ]	$t_g$ [m]	UK LS [m]
13.50	0.50	198.5	99.2	139.3	0.62 *	34.9 **	0.00	10.00	5.00	3.83	0.95
13.50	1.00	135.8	135.8	95.3	0.66 *	29.9 **	0.00	9.78	5.00	4.09	1.59
13.50	1.50	126.3	189.4	88.6	0.79 *	27.9 **	0.00	9.60	5.00	4.61	2.22
13.50	2.00	130.1	260.3	91.3	0.98 *	27.0 **	0.00	9.48	5.00	5.27	2.86
13.50	2.50	225.5	563.7	158.2	2.35 *	29.8 **	0.00	9.40	5.00	8.01	3.94
13.50	3.00	260.9	782.8	183.1	3.08 *	30.0 **	0.00	9.44	5.00	9.31	4.75
13.50	3.50	293.5	1027.3	206.0	3.80 *	30.0 **	0.00	9.49	5.00	10.50	5.55
13.50	4.00	322.3	1289.3	226.2	4.49 *	30.0 **	0.00	9.53	5.00	11.58	6.33
13.50	4.50	352.5	1586.1	247.3	5.22 *	30.0 **	0.00	9.57	5.00	12.65	7.12
13.50	5.00	382.9	1914.6	268.7	5.99 *	30.0 **	0.00	9.60	5.00	13.68	7.92
13.50	5.50	410.1	2255.7	287.8	6.72 *	30.0 **	0.00	9.63	5.00	14.62	8.71
13.50	6.00	440.0	2640.1	308.8	7.51 *	30.0 **	0.00	9.65	5.00	15.58	9.51
13.50	6.50	461.7	3001.3	324.0	8.17 *	29.9 **	0.00	9.67	5.00	16.37	10.27
13.50	7.00	487.3	3410.8	341.9	8.91 *	29.9 **	0.00	9.69	5.00	17.21	11.06
13.50	7.50	500.0	3750.0	350.9	9.39 *	30.0 **	0.00	9.71	5.00	17.82	11.88
13.50	8.00	500.0	4000.0	350.9	9.59 *	30.0 **	0.00	9.73	5.00	18.22	12.66

\* Vorbelastung = 30.0 kN/m<sup>2</sup>

\*\* phi wegen 5° Bedingung abgemindert

$\sigma_{E,k} = \sigma_{of,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{of,k} / (1.40 \cdot 1.43) = \sigma_{of,k} / 1.99$  (für Setzungen)

Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50

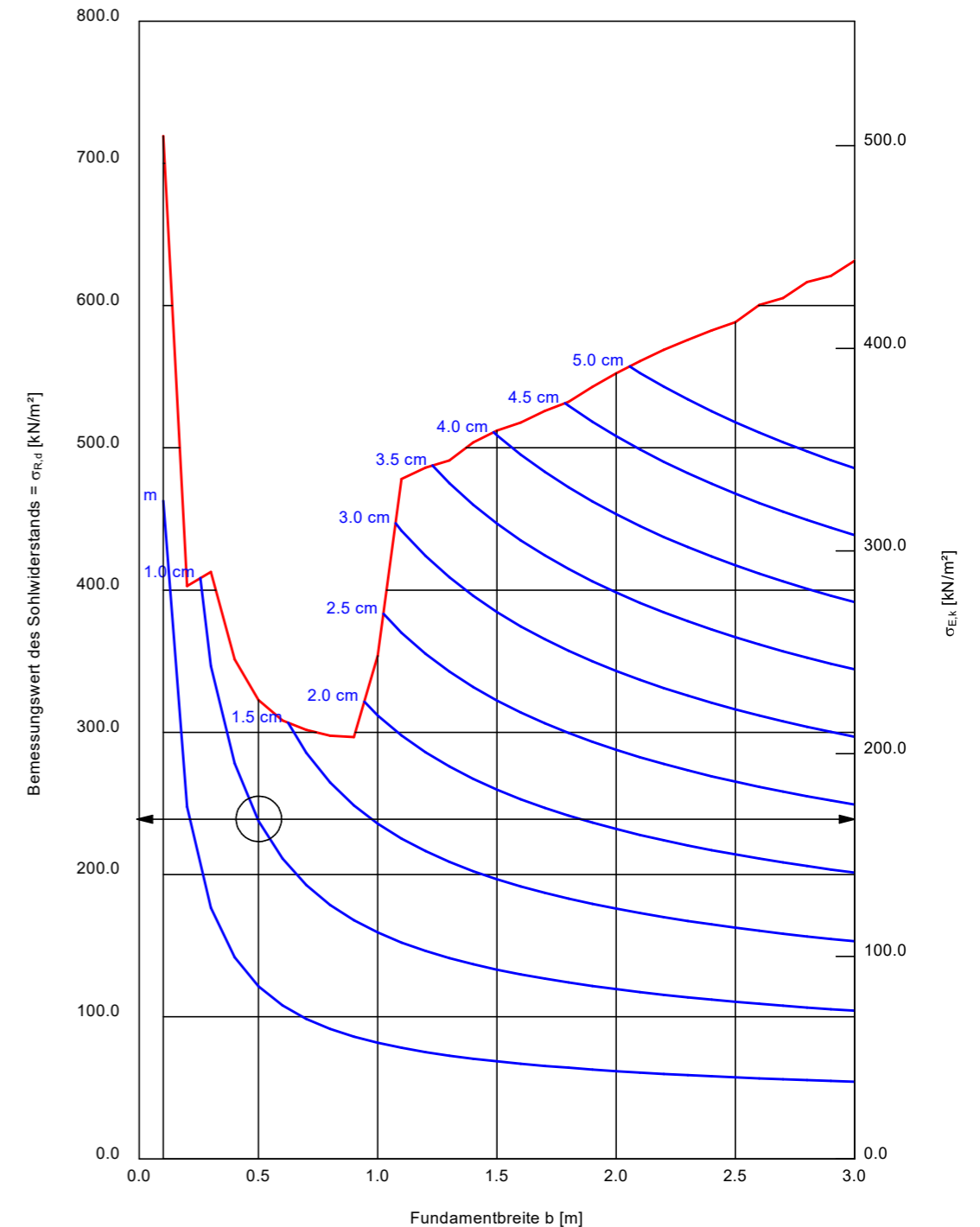
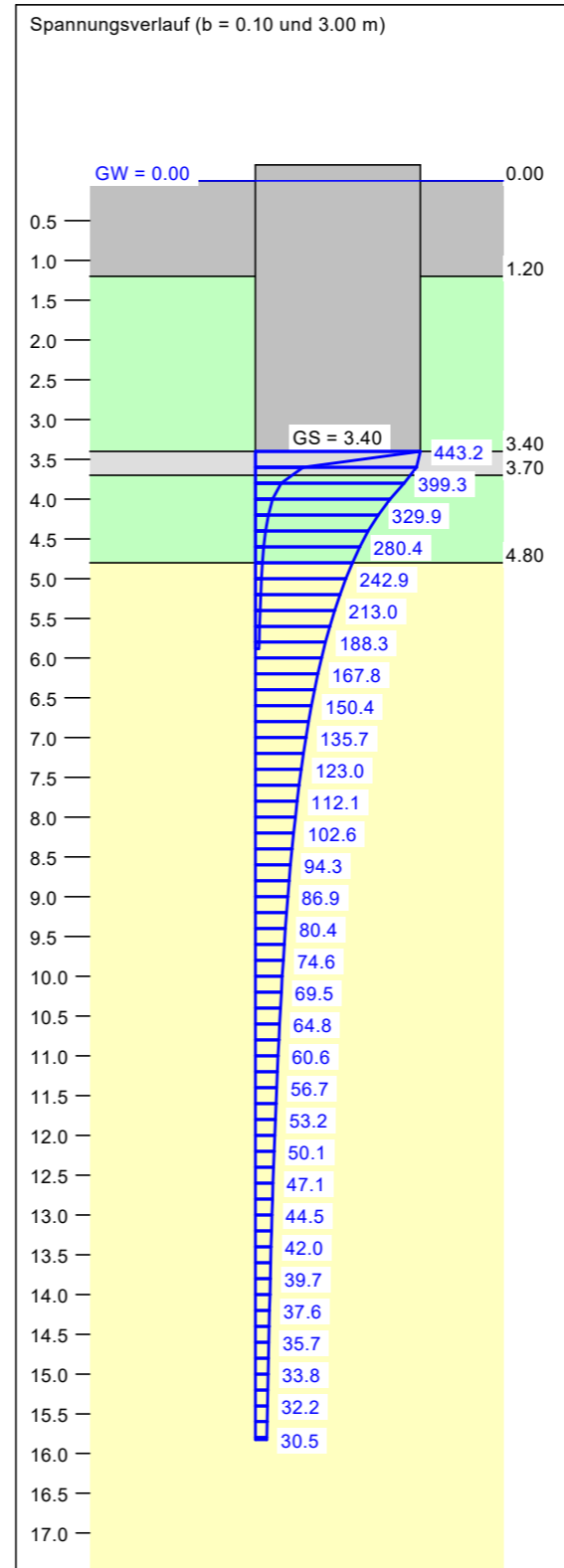
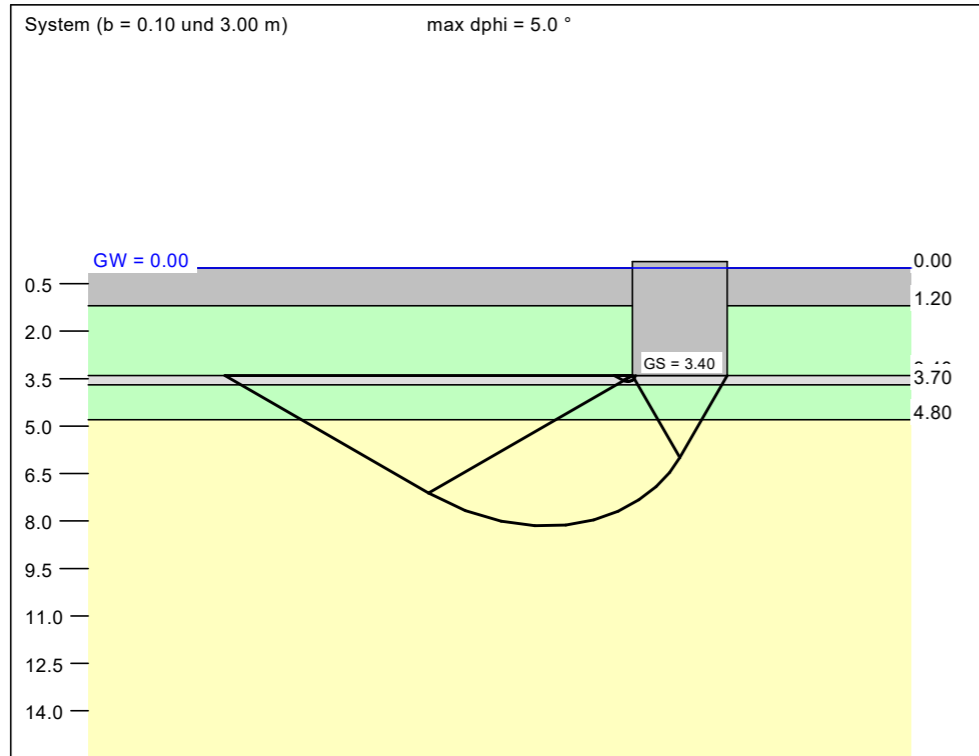


Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\varphi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
	18.0	8.0	22.5	0.0	2.0	0.00	Auffüllung
	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer Schluff
	20.0	10.0	35.0	0.0	80.0	0.00	Bodenersatzkörper
	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer Schluff
	20.0	10.0	35.0	0.0	80.0	0.00	Kies

Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 10.00 m)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 Gründungssohle = 3.40 m  
 Grundwasser = 0.00 m  
 Grenztiefe mit  $p = 20.0\%$   
 Grenztiefen spannungsvariabel bestimmt

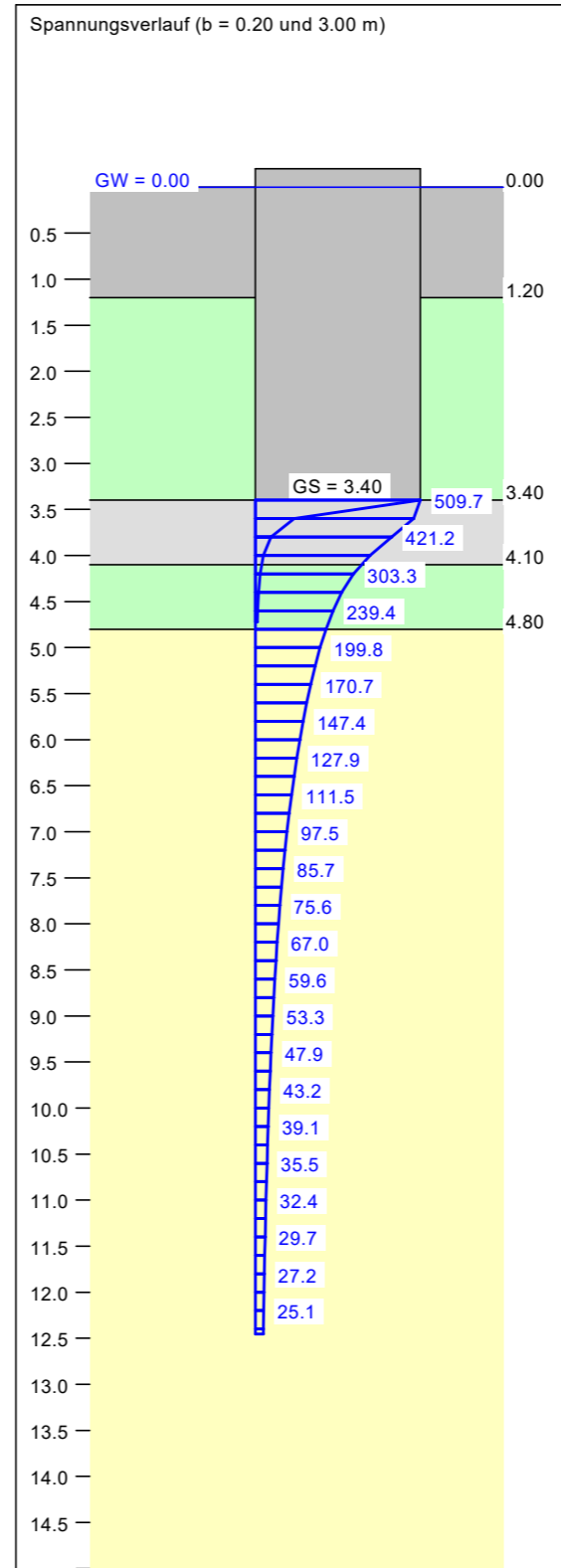
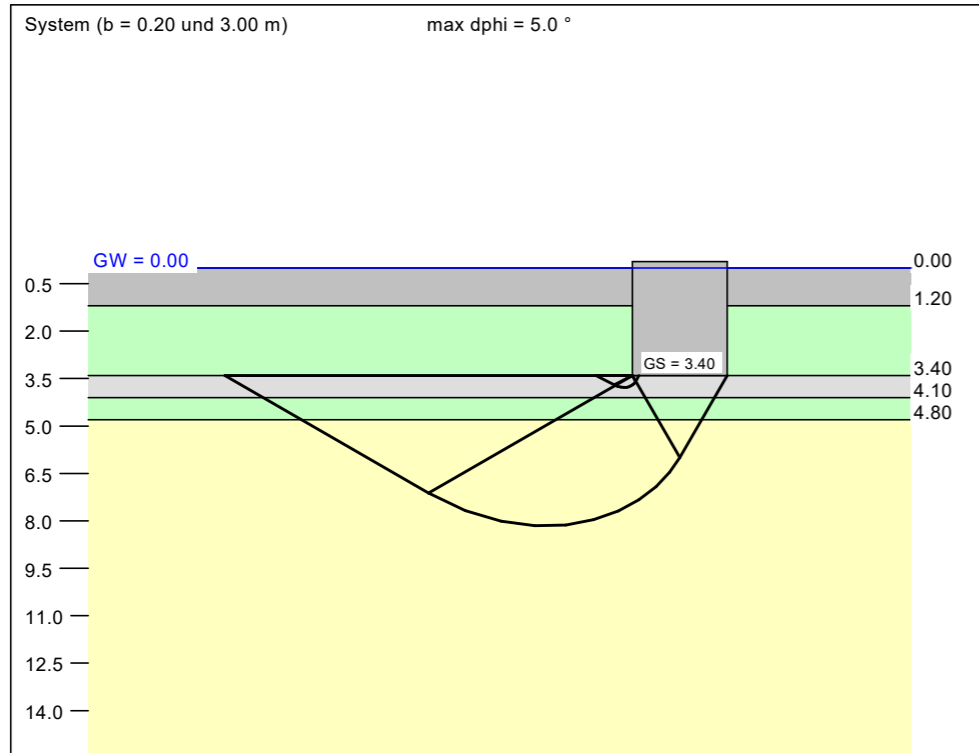
— Sohlendruck  
 — Setzungen



a [m]	b [m]	$\sigma_{R,d}$ [kN/m <sup>2</sup> ]	$R_{n,d}$ [kN/m]	$\sigma_{E,k}$ [kN/m <sup>2</sup> ]	s [cm]	cal $\varphi$ [°]	cal c [kN/m <sup>2</sup> ]	$\gamma_2$ [kN/m <sup>3</sup> ]	$\sigma_0$ [kN/m <sup>2</sup> ]	$t_g$ [m]	UK LS [m]
10.00	0.10	719.3	71.9	504.8	0.79	35.0	0.00	10.00	29.40	5.88	3.59
10.00	0.20	402.8	80.6	282.7	0.82	30.0*	0.00	9.99	29.40	6.05	3.72
10.00	0.30	412.8	123.9	289.7	1.20	30.0*	0.00	9.78	29.40	6.76	3.88
10.00	0.40	351.3	140.5	246.5	1.27	28.4*	0.00	9.65	29.40	7.00	4.00
10.00	0.50	322.7	161.4	226.5	1.37	27.5*	0.00	9.56	29.40	7.27	4.13
10.00	0.60	308.3	185.0	216.4	1.47	27.0*	0.00	9.48	29.40	7.56	4.26
10.00	0.70	301.7	211.2	211.7	1.59	26.6*	0.00	9.43	29.40	7.85	4.39
10.00	0.80	297.6	238.1	208.9	1.69	26.4*	0.00	9.38	29.40	8.14	4.52
10.00	0.90	296.6	266.9	208.1	1.80	26.2*	0.00	9.35	29.40	8.42	4.66
10.00	1.00	353.9	353.9	248.4	2.28	27.6*	0.00	9.31	29.40	9.20	4.86
10.00	1.10	478.2	526.0	335.5	3.25	30.0*	0.00	9.34	29.40	10.45	5.15
10.00	1.20	486.2	583.4	341.2	3.45	30.0*	0.00	9.37	29.40	10.81	5.30
10.00	1.30	491.2	638.6	344.7	3.62	29.9*	0.00	9.40	29.40	11.13	5.46
10.00	1.40	503.7	705.2	353.5	3.84	30.0*	0.00	9.42	29.40	11.49	5.62
10.00	1.50	512.1	768.1	359.4	4.03	30.0*	0.00	9.45	29.40	11.81	5.78
10.00	1.60	517.7	828.3	363.3	4.19	30.0*	0.00	9.47	29.40	12.11	5.93
10.00	1.70	525.9	894.0	369.0	4.36	29.9*	0.00	9.50	29.40	12.41	6.09
10.00	1.80	532.4	958.3	373.6	4.52	29.9*	0.00	9.52	29.40	12.69	6.25
10.00	1.90	542.8	1031.4	380.9	4.72	30.0*	0.00	9.54	29.40	13.00	6.41
10.00	2.00	552.3	1104.7	387.6	4.90	30.0*	0.00	9.55	29.40	13.29	6.57
10.00	2.10	561.0	1178.1	393.7	5.08	30.0*	0.00	9.57	29.40	13.57	6.73
10.00	2.20	568.9	1251.5	399.2	5.25	30.0*	0.00	9.59	29.40	13.84	6.89
10.00	2.30	576.0	1324.8	404.2	5.40	30.0*	0.00	9.60	29.40	14.10	7.04
10.00	2.40	582.5	1398.1	408.8	5.56	29.9*	0.00	9.61	29.40	14.35	7.20
10.00	2.50	588.4	1471.1	412.9	5.70	29.9*	0.00	9.63	29.40	14.59	7.35
10.00	2.60	600.4	1560.9	421.3	5.91	30.0*	0.00	9.64	29.40	14.87	7.52
10.00	2.70	605.3	1634.4	424.8	6.04	29.9*	0.00	9.65	29.40	15.09	7.67
10.00	2.80	616.7	1726.6	432.7	6.24	30.0*	0.00	9.66	29.40	15.36	7.84
10.00	2.90	620.8	1800.3	435.6	6.36	29.9*	0.00	9.67	29.40	15.57	7.99
10.00	3.00	631.6	1894.8	443.2	6.56	30.0*	0.00	9.68	29.40	15.82	8.15

\* phi wegen 5° Bedingung abgemindert  
 $\sigma_{E,k} = \sigma_{0f,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{0f,k} / (1.40 \cdot 1.43) = \sigma_{0f,k} / 1.99$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50

Boden	$\gamma$ [kN/m <sup>3</sup> ]	$\gamma'$ [kN/m <sup>3</sup> ]	$\varphi$ [°]	c [kN/m <sup>2</sup> ]	$E_s$ [MN/m <sup>2</sup> ]	$\nu$ [-]	Bezeichnung
[Grey]	18.0	8.0	22.5	0.0	2.0	0.00	Auffüllung
[Light Green]	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer Schluff
[Light Grey]	20.0	10.0	35.0	0.0	80.0	0.00	Bodenersatzkörper
[Light Green]	19.0	9.0	25.0	0.0	7.0	0.00	Schwemmfächer Schluff
[Yellow]	20.0	10.0	35.0	0.0	80.0	0.00	Kies



Berechnungsgrundlagen:  
 Norm: EC 7  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Einzelfundament (a/b = 1.00)  
 $\gamma_{R,v} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.425$   
 Gründungssohle = 3.40 m  
 Grundwasser = 0.00 m  
 Grenztiefe mit  $p = 20.0\%$   
 Grenztiefen spannungsvariabel bestimmt

— Sohldruck  
 — Setzungen

a [m]	b [m]	$\sigma_{R,d}$ [kN/m <sup>2</sup> ]	$R_{n,d}$ [kN]	$\sigma_{E,k}$ [kN/m <sup>2</sup> ]	s [cm]	cal $\varphi$ [°]	cal c [kN/m <sup>2</sup> ]	$\gamma_2$ [kN/m <sup>3</sup> ]	$\sigma_{\bar{0}}$ [kN/m <sup>2</sup> ]	$t_g$ [m]	UK LS [m]
0.20	0.20	1122.9	44.9	788.0	0.28	35.0	0.00	10.00	29.40	4.72	3.78
0.30	0.30	1134.2	102.1	795.9	0.52	35.0	0.00	10.00	29.40	5.27	3.97
0.40	0.40	845.3	135.3	593.2	0.58	32.7*	0.00	10.00	29.40	5.49	4.10
0.50	0.50	600.9	150.2	421.7	0.56	29.9*	0.00	9.96	29.40	5.57	4.19
0.60	0.60	606.3	218.3	425.5	0.71	30.0*	0.00	9.87	29.40	5.93	4.35
0.70	0.70	613.6	300.7	430.6	0.87	30.0*	0.00	9.78	29.40	6.27	4.51
0.80	0.80	563.5	360.6	395.4	0.93	29.3*	0.00	9.72	29.40	6.48	4.64
0.90	0.90	609.5	493.7	427.7	1.15	29.8*	0.00	9.65	29.40	6.88	4.82
1.00	1.00	625.1	625.1	438.7	1.32	30.0*	0.00	9.63	29.40	7.22	4.98
1.10	1.10	628.5	760.5	441.1	1.46	29.9*	0.00	9.63	29.40	7.51	5.14
1.20	1.20	635.8	915.5	446.2	1.60	30.0*	0.00	9.64	29.40	7.81	5.30
1.30	1.30	642.1	1085.2	450.6	1.74	30.0*	0.00	9.65	29.40	8.11	5.46
1.40	1.40	645.3	1264.8	452.9	1.86	30.0*	0.00	9.66	29.40	8.38	5.62
1.50	1.50	645.7	1452.9	453.1	1.97	29.9*	0.00	9.68	29.40	8.65	5.77
1.60	1.60	651.5	1667.8	457.2	2.10	29.9*	0.00	9.69	29.40	8.92	5.93
1.70	1.70	655.9	1895.5	460.3	2.22	29.9*	0.00	9.70	29.40	9.19	6.09
1.80	1.80	666.7	2160.0	467.8	2.36	30.0*	0.00	9.71	29.40	9.48	6.25
1.90	1.90	669.4	2416.7	469.8	2.47	30.0*	0.00	9.72	29.40	9.73	6.41
2.00	2.00	671.5	2686.0	471.2	2.58	29.9*	0.00	9.73	29.40	9.98	6.56
2.10	2.10	680.6	3001.6	477.6	2.71	30.0*	0.00	9.74	29.40	10.25	6.73
2.20	2.20	681.6	3299.0	478.3	2.81	29.9*	0.00	9.75	29.40	10.49	6.88
2.30	2.30	690.0	3650.3	484.2	2.94	30.0*	0.00	9.76	29.40	10.75	7.04
2.40	2.40	690.2	3975.4	484.3	3.03	29.9*	0.00	9.76	29.40	10.98	7.19
2.50	2.50	698.0	4362.7	489.8	3.16	30.0*	0.00	9.77	29.40	11.24	7.36
2.60	2.60	705.7	4770.6	495.2	3.29	30.0*	0.00	9.78	29.40	11.49	7.52
2.70	2.70	704.9	5138.4	494.6	3.38	29.9*	0.00	9.79	29.40	11.71	7.67
2.80	2.80	712.1	5583.1	499.7	3.50	29.9*	0.00	9.79	29.40	11.96	7.83
2.90	2.90	719.3	6049.1	504.8	3.63	30.0*	0.00	9.80	29.40	12.21	7.99
3.00	3.00	726.3	6536.8	509.7	3.75	30.0*	0.00	9.80	29.40	12.46	8.15

\* phi wegen 5° Bedingung abgemindert  
 $\sigma_{E,k} = \sigma_{0f,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{0f,k} / (1.40 \cdot 1.43) = \sigma_{0f,k} / 1.99$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50

